

Lecture 12

STR403 - Metallic Bridges “Box-Girder Bridges”

Sherif A. Mourad

M. Hassanien

Professors of Steel Structures and Bridges

Faculty of Engineering, Cairo University

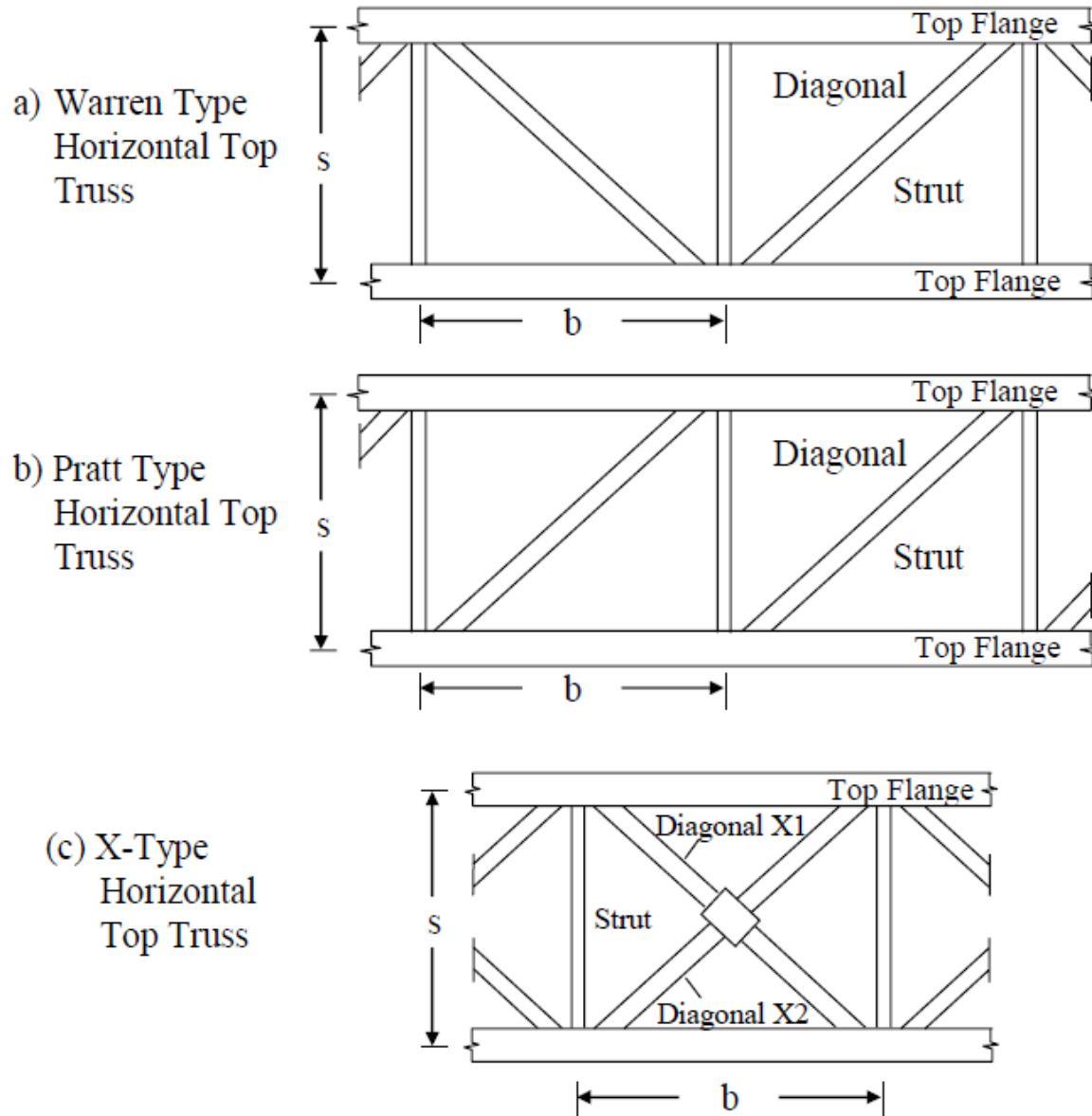
Lecture 12: Box-Girder Bridges

Topics

- Behavior of Box-Girder (Continue).
 - Top Flange Truss System

Behavior of Box-Girder

Geometry for Top Flange Lateral Trusses

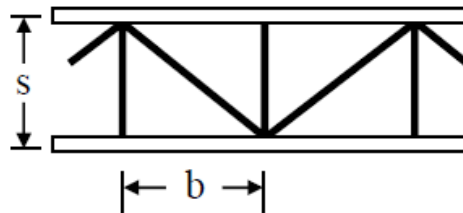


Behavior of Box-Girder

Equivalent Thickness of Top Flange Truss System

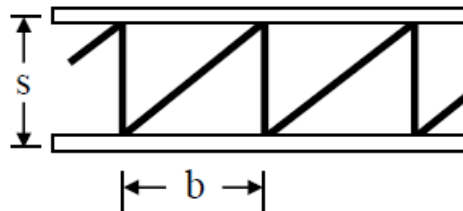
t^* = equivalent plate thickness;
 E = Modulus of elasticity (29000 ksi for steel);
 G = Shear modulus (11200 ksi for steel);
 b = panel length (spacing between struts);
 s = strut length (width between flanges);
 L_d = diagonal length $\sqrt{\quad}$
 A_d = area of diagonal;
 A_s = area of strut;
 A_f = area of girder top flange (one flange).

a) Warren Type
Horizontal Top
Truss



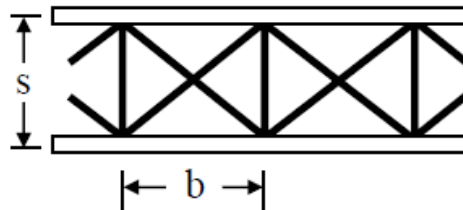
$$t^* = \frac{E}{G} \frac{sb}{\left[\frac{L_d^3}{A_d} + \frac{s^3}{3} \left(\frac{2}{A_f} \right) \right]}$$

b) Pratt Type
Horizontal Top
Truss



$$t^* = \frac{E}{G} \frac{sb}{\left[\frac{L_d^3}{A_d} + \frac{b^3}{A_s} + \frac{s^3}{12} \left(\frac{2}{A_f} \right) \right]}$$

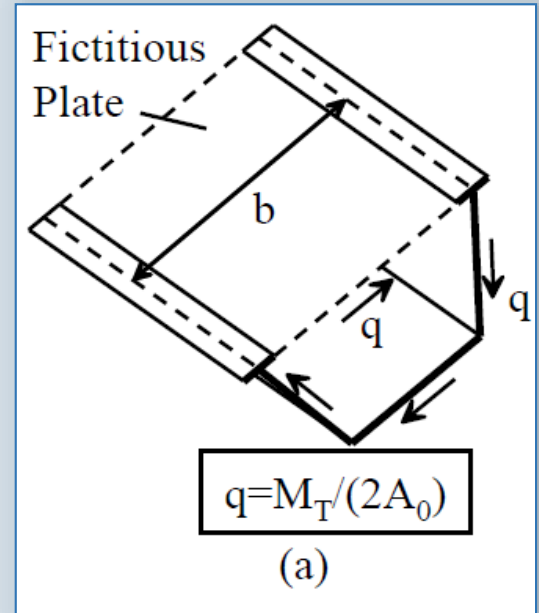
c) X-Type
Horizontal
Top Truss



$$t^* = \frac{E}{G} \frac{sb}{\left[\frac{L_d^3}{2A_d} + \frac{s^3}{12} \left(\frac{2}{A_f} \right) \right]}$$

EPM

Equivalent Plate Method

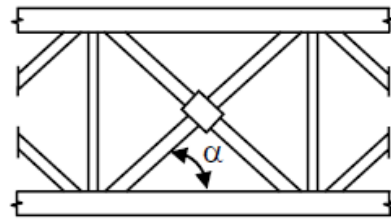
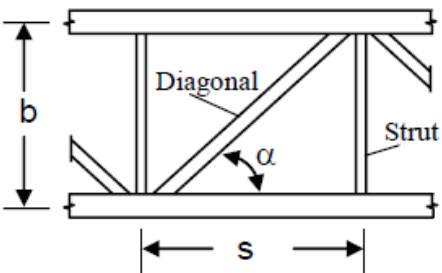


Behavior of Box-Girder

Forces in Top Flange Truss System

1 Bending Induced Forces

D_{bend} = bending component in diagonal
 S_{bend} = bending component in strut
 $f_{x \text{ top}}$ = top flange bending stress in panel
 s = panel length (spacing between struts)
 α = Angle between diagonal and flange
 L_d = diagonal length
 b = strut length (width between flanges)
 A_d, A_s = respective area of diagonal or strut
 b_f, t_f = respective width and thickness of girder flange
 $f_{L \text{ bend}}$ = lateral bending stress in girder top flange



$$D_{\text{bend}} = \frac{f_{x \text{ Top}} s \cos \alpha}{K_1}$$

$$K_1 = \frac{L_d}{A_d} + \frac{b}{A_s} \sin^2 \alpha + \frac{s^3}{2b_f^3 t_f} \sin^2 \alpha$$

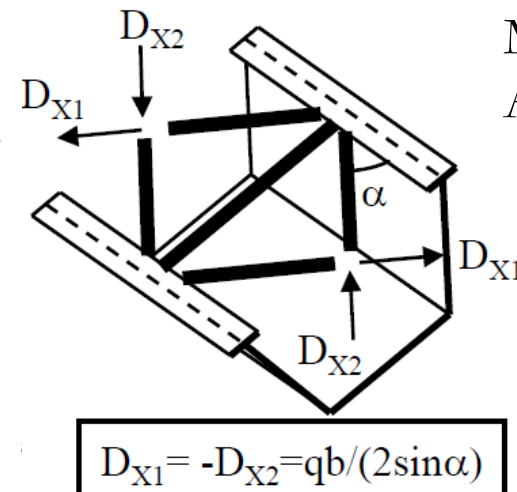
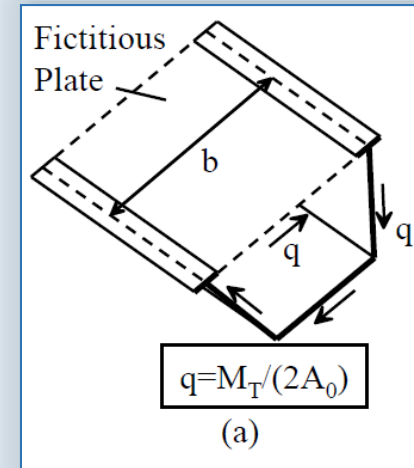
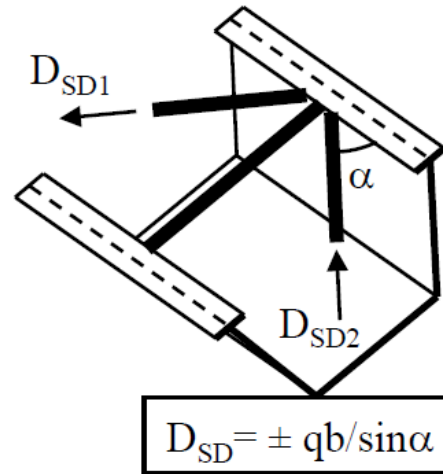
$$S_{\text{bend}} = -D_{\text{bend}} \sin \alpha$$

$$D_{\text{bend}} = \frac{f_{x \text{ Top}} s \cos \alpha}{K_2}$$

$$K_2 = \frac{L_d}{A_d} + \frac{2b \sin^2 \alpha}{A_s}$$

$$S_{\text{bend}} = -2D_{\text{bend}} \sin \alpha$$

2 Torsion Induced Forces

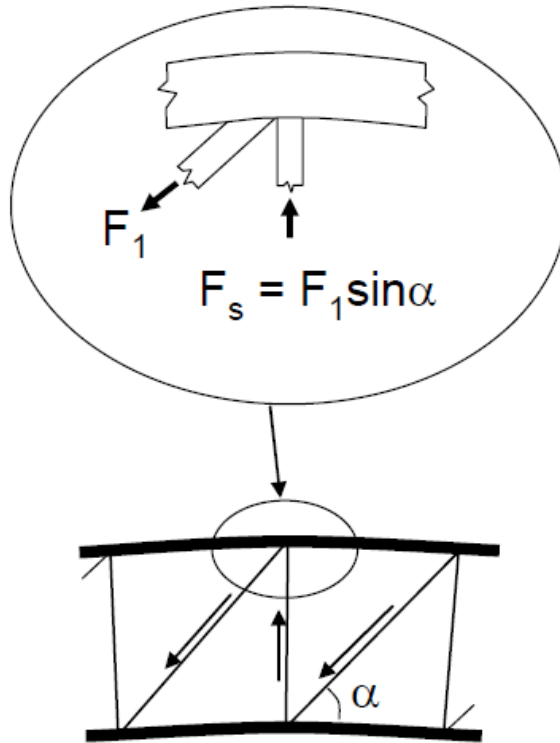


q = Shear Flow
 M_T = Torsion Moment
 $A_0 = \sum b_i t_i$

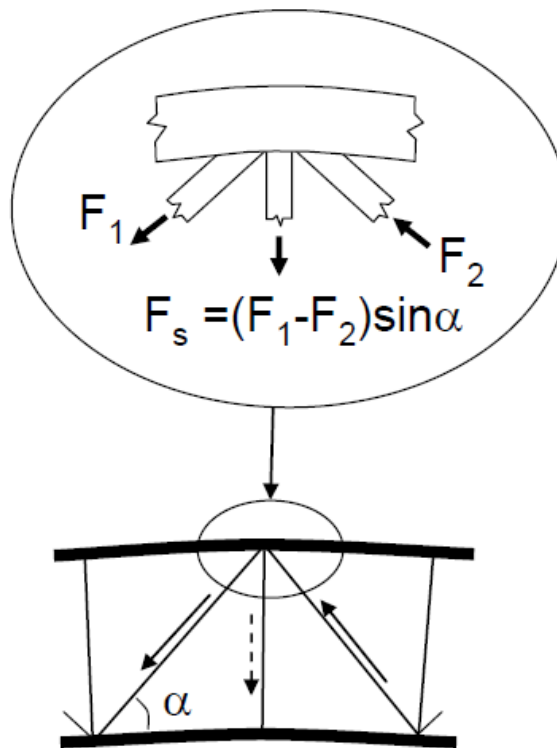
Superposition:
 $D_{\text{bend}} + D_{SD}$
 $D_{\text{bend}} + D_X$

Behavior of Box-Girder

Forces in Top Flange Truss System



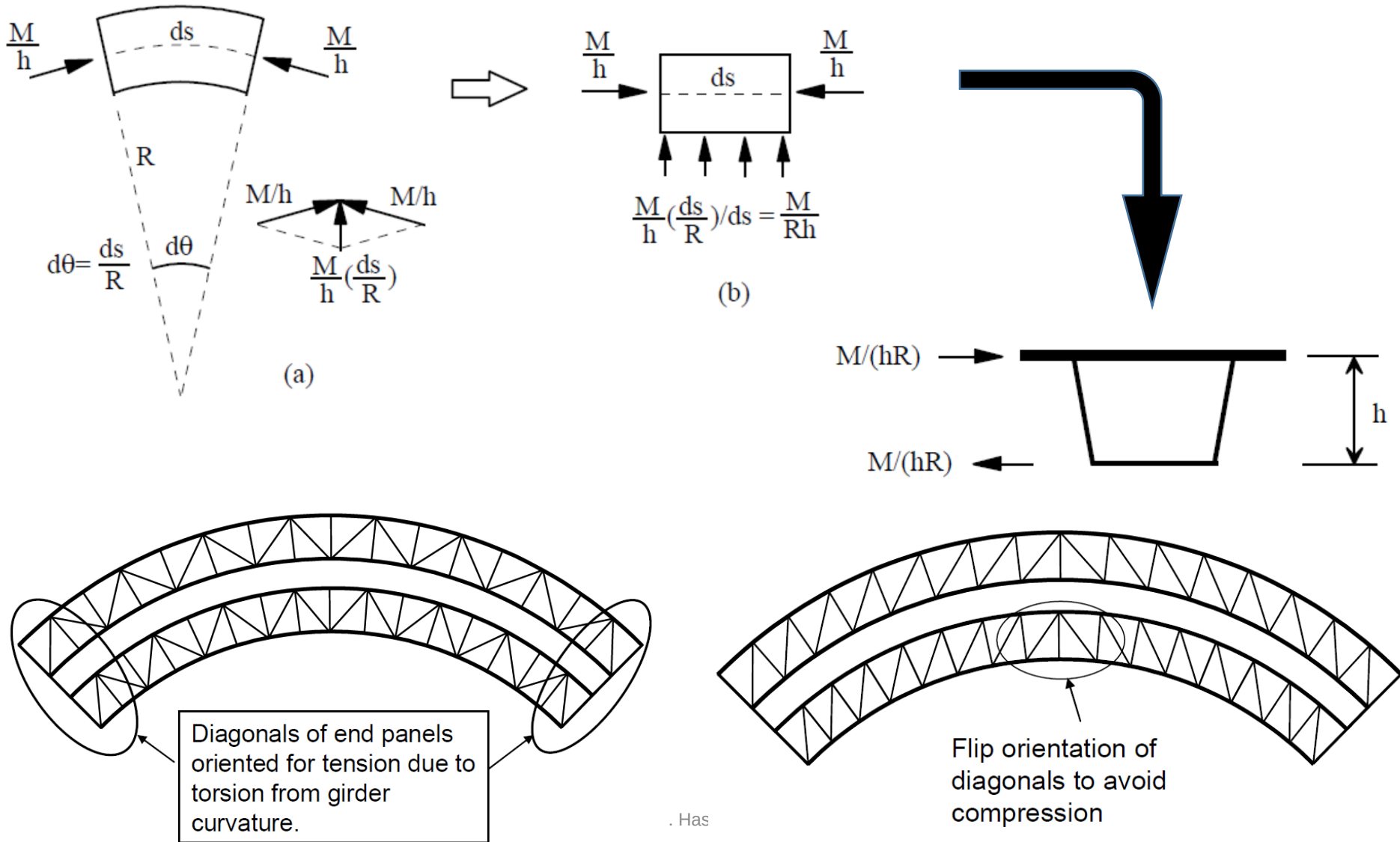
(a) Pratt Truss



(b) Warren Truss

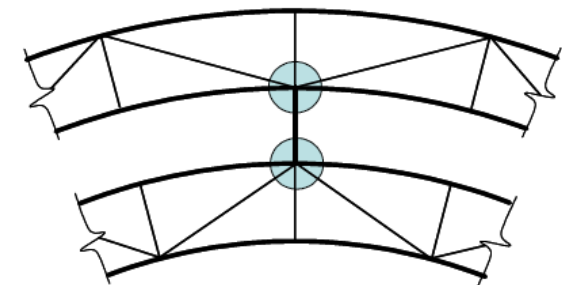
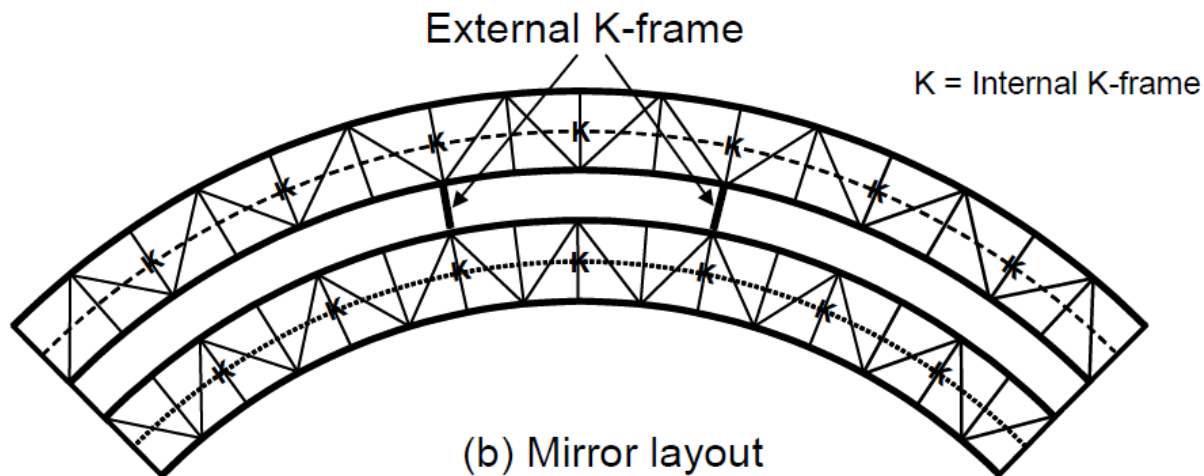
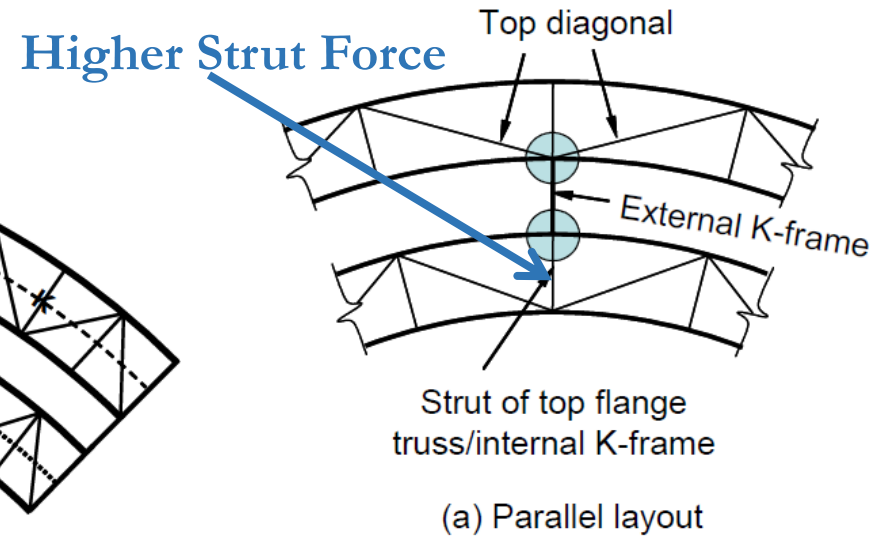
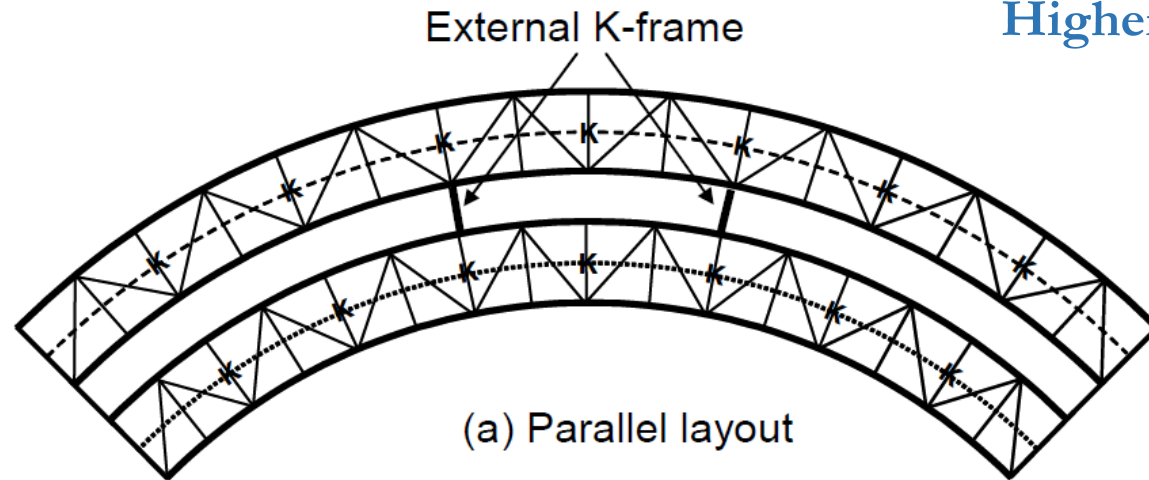
Behavior of Box-Girder

Forces in Top Flange Truss System



Behavior of Box-Girder

Forces in Top Flange Truss System

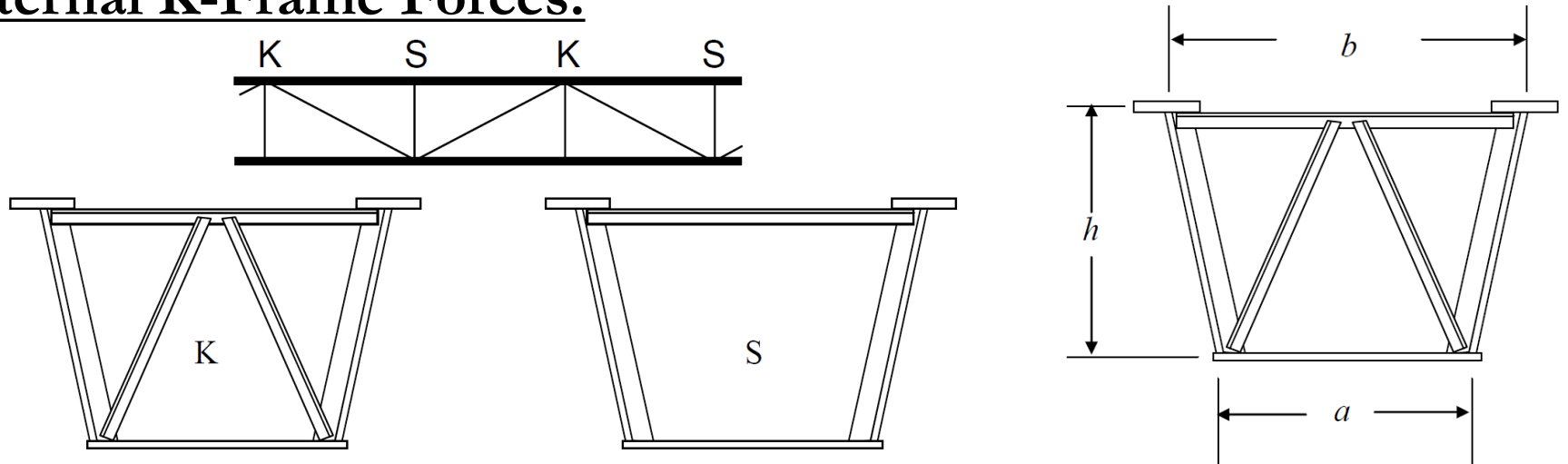


Parallel Versus Mirrored Layout of Top Flange Lateral Truss

(b) Mirror layout

Behavior of Box-Girder

Internal K-Frame Forces:



D = distortional induced force in the K-frame diagonal;

S = distortional induced force in the K-frame strut;

s_K = spacing between internal K-frames measured along the girder length;

L_{DK} = length of the K-frame diagonal;

A_0 = area enclosed by box = $(a+b)/2h$;

a and b = box girder dimensions as depicted in Figure 4.5a;

e = effective eccentricity of resultant distributed load;

w = distributed load (weight/ft.);

M = box girder bending moment; and

R = radius of horizontal curvature of box.

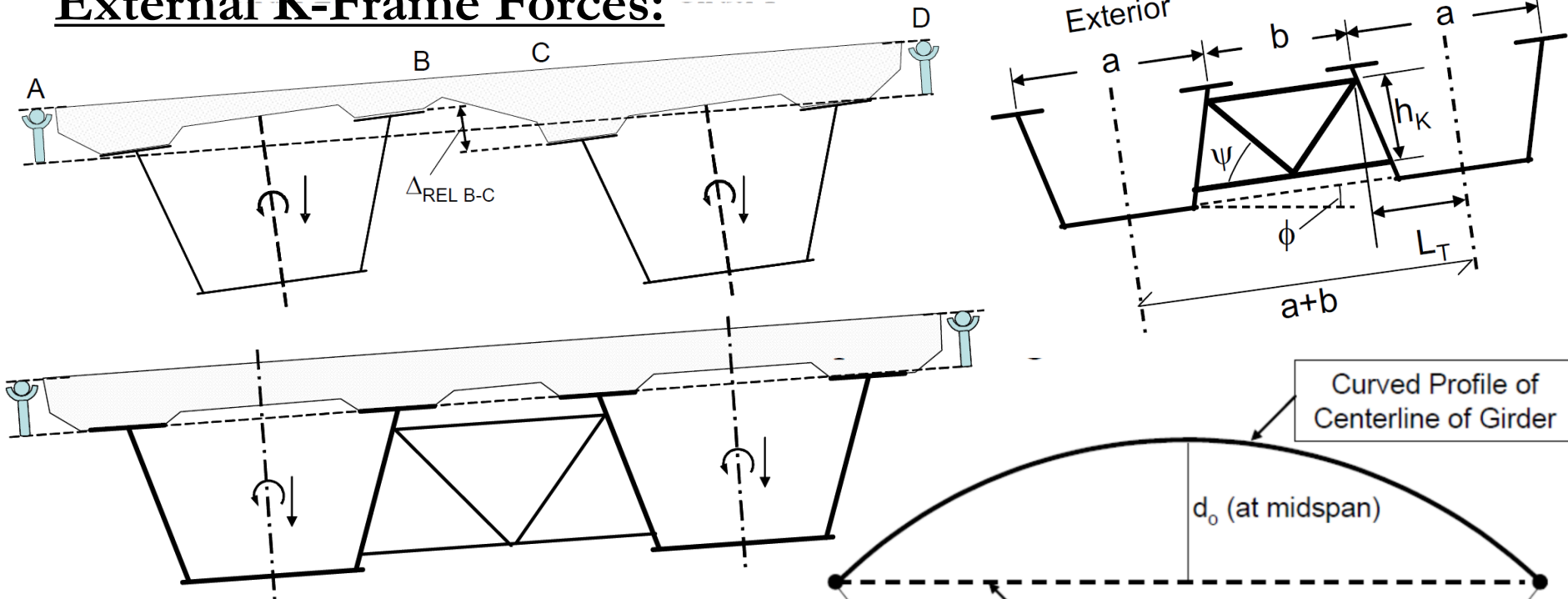
$$D = \pm \frac{s_K L_{DK}}{2A_0} \left(\frac{M}{R} - \frac{a}{b} ew \right)$$

M. Hassanien

$$S = \pm \frac{s_K a}{4A_0} \left(\frac{a}{b} ew - \frac{M}{R} \right)$$

Behavior of Box-Girder

External K-Frame Forces:



L_e = Span of Exterior Girder,

L_i = Span of Interior Girder,

β_o = Subtended Angle,

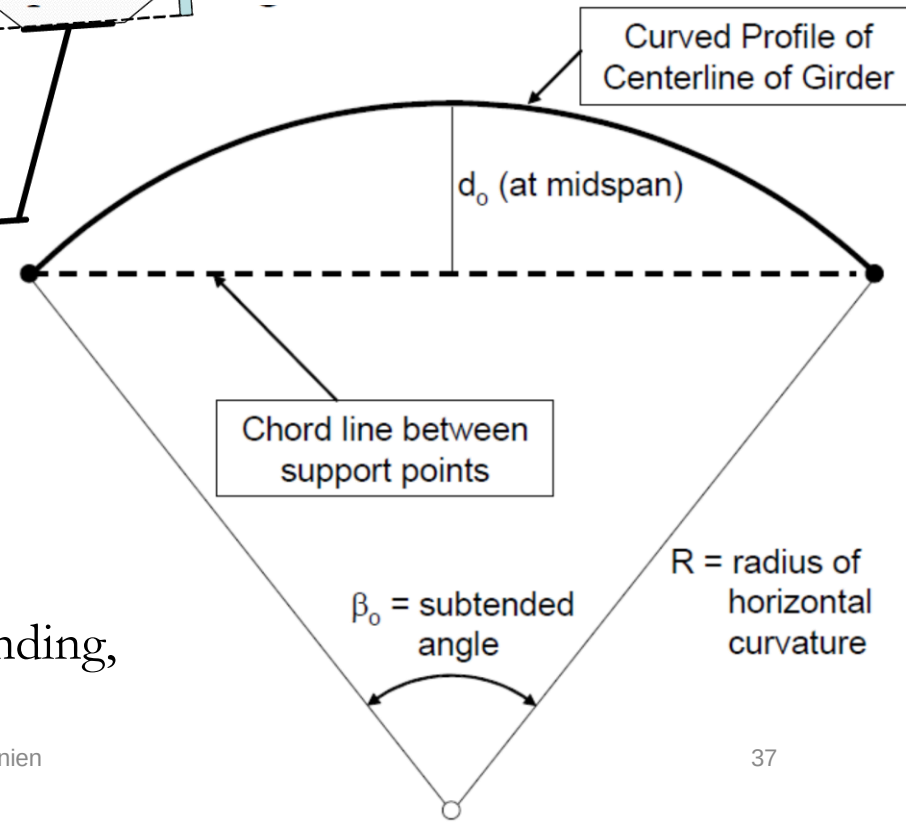
R = Horizontal Radius of Curvature,

E = Modulus of Elasticity,

I = Moment of inertia about the axis of bending,

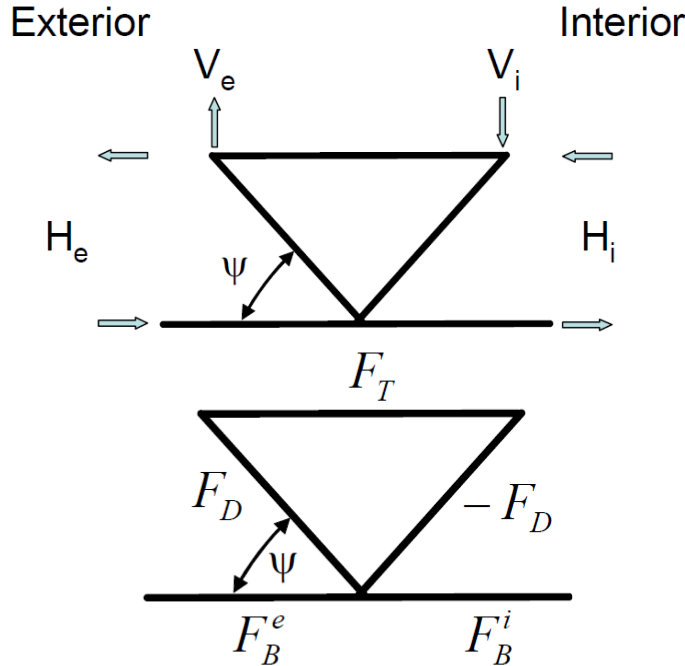
G = Shear Modulus,

J = Torsional Constant = $1/3 \sum b_i t_i^3$



Behavior of Box-Girder

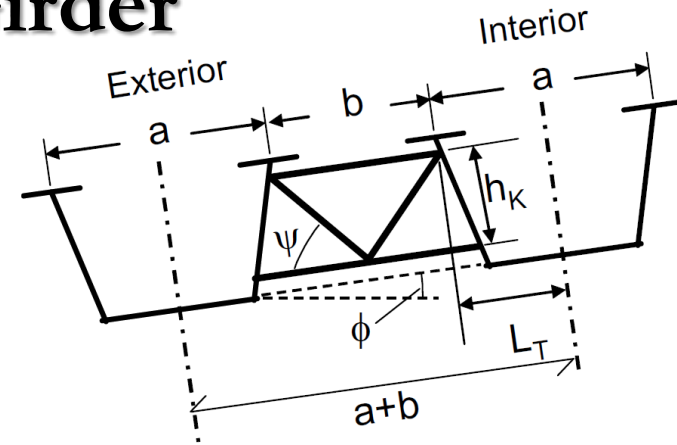
External K-Frame Forces:



$$F_D = 4GJ \frac{L_i \phi_{w,ext} + L_e \phi_{w,int} - K_1 \Delta_{w,rel}}{K_2}$$

$$F_T = \frac{4GJ(\phi_{w,ext} - \phi_{w,int}) - F_D L_K (L_e - L_i)}{h_K (L_i + L_e)}$$

$$F_B = \pm F_D \cos \psi - F_T$$



$$L_K = h_K \cos \psi + L_T \sin \psi$$

$$K_0 = 1 + \left(1 + \frac{EI}{GJ}\right) \left(1 - \cos \frac{\beta_0}{2}\right)$$

$$K_1 = \frac{L_i + L_e}{a + b}$$

$$K_2 = K_0 K_1 \frac{L_i^3 + L_e^3}{12(EI / GJ)} \sin \psi + 2L_i L_e L_K$$

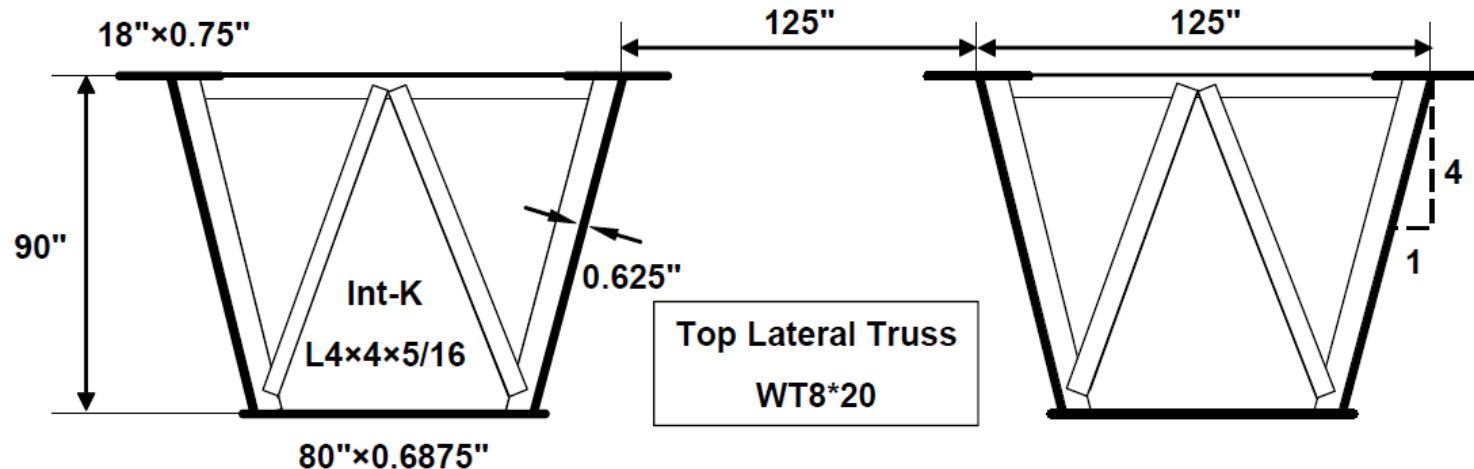
$$\Delta_{w,rel} = K_0 \frac{5w}{384EI} (L_e^4 - L_i^4)$$

$$\phi_{w,int} = \frac{5wL_i^4}{384EIR_{int}} \left(1 + \frac{EI}{GJ}\right)$$

$$\phi_{w,ext} = \frac{5wL_e^4}{384EIR_{ext}} \left(1 + \frac{EI}{GJ}\right)$$

Behavior of Box-Girder

External K-Frame (Example):



$$J = 97,000 \text{ inch}^4 \text{ And}$$

$$I = 234,000 \text{ inch}^4$$

$$\beta_0 = \frac{L}{R} = \frac{160}{600} = 0.2667 \text{ rad} = 15.28^\circ$$

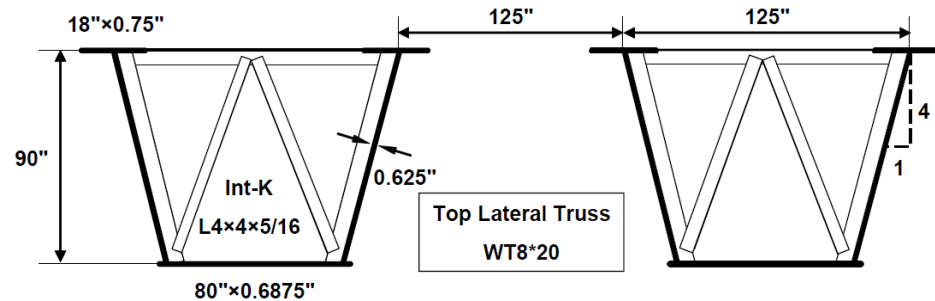
$$\frac{EI}{GJ} = \frac{29000 \times 234000}{112000 \times 97000} = 6.3$$

$$a = b = 10.4'$$

$$L_i = L \left(\frac{R - b}{R} \right) = 160 \times \left(\frac{600 - 10.4}{600} \right) = 157.22'$$

Behavior of Box-Girder

External K-Frame (Example):



$$\Delta_{w,int} \left(\frac{L}{2} \right) = \frac{5wL_i^4}{384EI} = \frac{5 \times 2 \times 157.22^4}{384 \times 29000 \times 234000 / 144} = 0.359' = 4.31''$$

$$\phi_{w,int} \left(\frac{L}{2} \right) = \frac{5wL_t^4}{384EIR_{int}} \left(1 + \frac{EI}{GJ} \right) = \frac{5 \times 2 \times 157.22^4}{384 \times 29000 \times 234000 / 144 \times 589.6} (1 + 6.3)$$

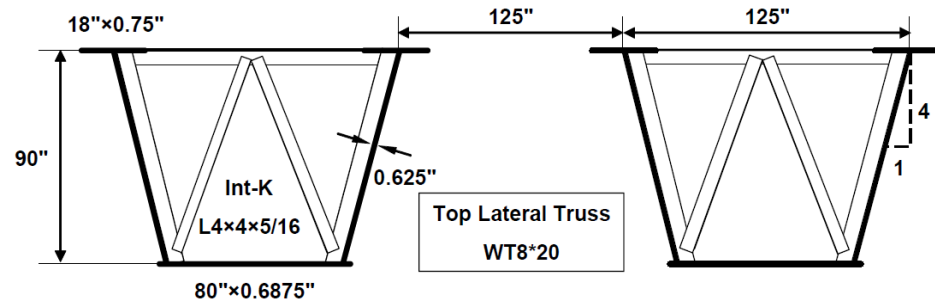
$$= 0.00417 \text{ rad}$$

$$d_0 = \frac{R_{int} + R_{ext}}{2} (1 - \cos \frac{\beta_0}{2}) = 600' \times (1 - \cos 7.64^\circ) = 64''$$

$$L_e = L \left(\frac{R+b}{R} \right) = 160 \times \left(\frac{600 + 10.4}{600} \right) = 610.4'$$

Behavior of Box-Girder

External K-Frame (Example):



$$\Delta_{w,ext} \left(\frac{L}{2} \right) = \frac{5wL_e^4}{384EI} = \frac{5 \times 2 \times 162.78^4}{384 \times 29000 \times 234000 / 144} = 0.413' = 4.95''$$

$$\phi_{w,ext} \left(\frac{L}{2} \right) = \frac{5wL_e^4}{384EI R_{ext}} \left(1 + \frac{EI}{GJ} \right) = \frac{5 \times 2 \times 162.78^4}{384 \times 29000 \times 234000 / 144 \times 610.4} (1 + 6.3)$$

$$= 0.00463 \text{ rad}$$

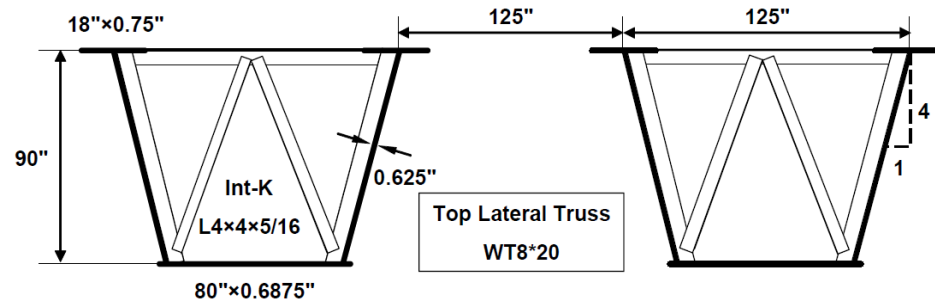
$$h_K = 0.6 \times 90'' = 54'' = 4.5'$$

Since the slope of web is $\frac{1}{4}$, L_T is

$$L_T = \frac{125''}{2} - 0.2 \times 90'' \times 0.25 = 58'' = 4.83'$$

Behavior of Box-Girder

External K-Frame (Example):



$$\psi = \arctan \left(\frac{h_K}{\frac{(b+a)}{2} - L_T} \right) = \arctan \left(\frac{54}{125 - 58} \right) = 38.9^\circ$$

$$L_K = h_K \cos \psi + L_T \sin \psi = 4.5 * \cos 38.9^\circ + 4.83 * \sin 38.9^\circ = 6.53'$$

$$K_0 = 1 + \left(1 + \frac{EI}{GJ} \right) \left(1 - \cos \frac{\beta_o}{2} \right) = 1 + (1 + 6.3) \left(1 - \cos \frac{15.27^\circ}{2} \right) = 1.065$$

$$K_1 = \frac{L_i + L_e}{a + b} = 15.36$$

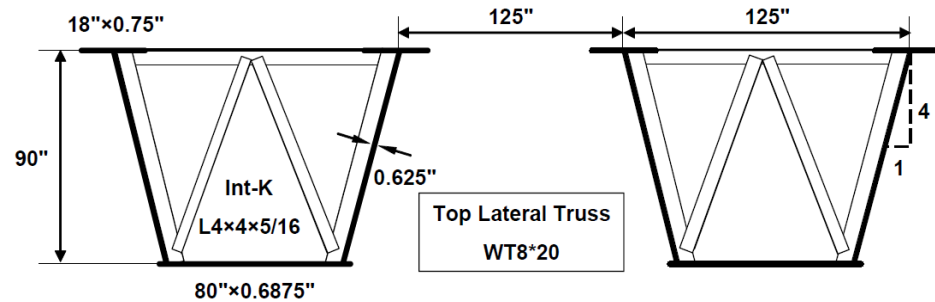
$$K_2 = K_0 K_1 \frac{L_i^3 + L_e^3}{12(EI / GJ)} \sin \psi + 2L_i L_e L_K$$

$$= 1.065 * 15.36 * \frac{157.2^3 + 162.8^3}{12 * 6.3} \sin 38.9^\circ + 2 * 157.2 * 162.8 * 6.53$$

$$= 1,448,000$$

Behavior of Box-Girder

External K-Frame (Example):



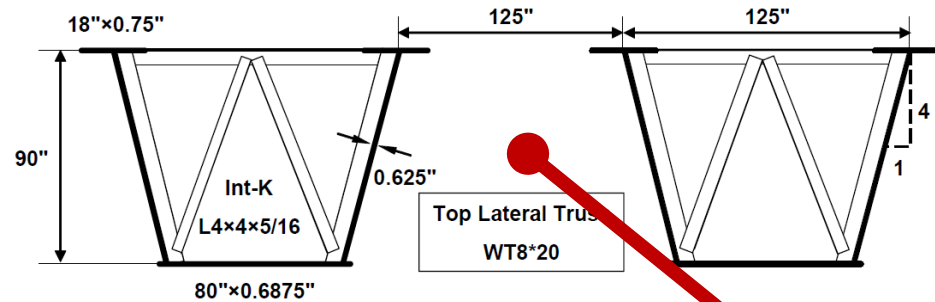
$$\Delta_w = K_0 \frac{5w}{384EI} (L_e^4 - L_i^4) = 1.065 * \frac{5 * 2.0}{384 * 29000 * 234000 / 144} (162.8^4 - 157.2^4) = 0.6"$$

$$\phi_{w,int} = \frac{5wL_i^4}{384EI R_{int}} \left(1 + \frac{EI}{GJ} \right) = \frac{5 * 2.0 * 157.2^4 * 7.3}{384 * 29000 * 234000 / 144 * 589.6} = 0.00418 \text{ rad}$$

$$\phi_{w,ext} = \frac{5wL_e^4}{384EI R_{ext}} \left(1 + \frac{EI}{GJ} \right) = 0.00463 \text{ rad}$$

Behavior of Box-Girder

External K-Frame (Example):



$$F_D = 4GJ \frac{L_i \phi_{w,ext} + L_e \phi_{w,int} - K_1 \Delta_w}{K_2}$$

$$= \frac{4 * 11200 * 97000}{144} \times \frac{157.2 * 0.00463 + 162.8 * 0.00418 - 15.36 * 0.6 / 12}{1448000}$$

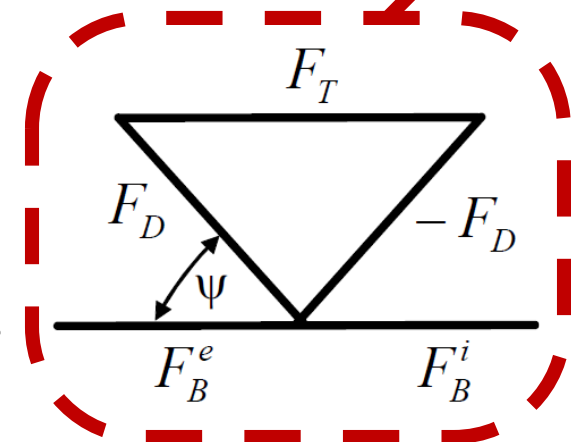
$$= 13.3 \text{ kips}$$

$$F_T = \frac{4GJ(\phi_{w,ext} - \phi_{w,int}) - F_D L_K (L_e - L_i)}{h'(L_i + L_e)}$$

$$= \frac{4GJ(0.00463 - 0.00418) - 13.3 * 6.53(162.8 - 157.2)}{4.5(320)}$$

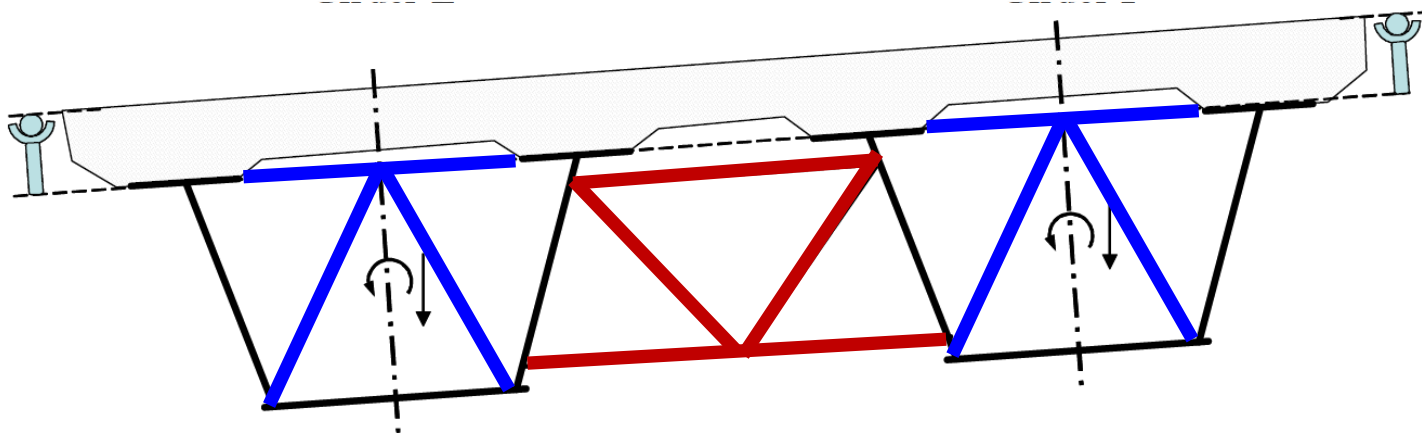
$$= 9.1 \text{ kips}$$

$$F_B = \pm F_D \cos \psi - F_T = \pm 13.3 * \cos 38.9^\circ - 9.1 = \begin{cases} +1.3 \\ -19.4 \end{cases} \text{ kips}$$



Behavior of Box-Girder

External and Internal K-Frames:



Lecture 12: Box-Girder Bridges

Summary of Today's Topics

- Behavior of Box-Girder (Continue).
 - Top Flange Truss System